#040523

VILLAGE OF LOMBARD
REQUEST FOR BOARD OF TRUSTEES ACTION
For Inclusion on Board Agenda

_____ Resolution or Ordinance (Blue) _____ *Waiver of First Requested*
_____ Recommendations of Boards, Commissions & Committees (Green)
___X___ Other Business (Pink)

TO: PRESIDENT AND BOARD OF TRUSTEES

FROM: William T. Lichter, Village Manager

DATE: October 25, 2004 (B of T) Date: November 4, 2004

TITLE: Yorktown Shopping Center Certification Letter

SUBMITTED BY: Department of Community Development

BACKGROUND/POLICY IMPLICATIONS:

The Department of Community Development transmits for your consideration correspondence to Long-Pehrson Associates relative to future stormwater improvements associated with the Yorktown Planned Development and its peripheral Planned Development. (DISTRICT #3)

Staff recommends approval of this request.

Please place this item on the November 4, 2004 Board of Trustees agenda.

Fiscal Impact/Funding Source:

Review (as necessary):
Village Attorney X _____ Date _____
Finance Director X _____ Date _____
Village Manager X W. Thomas Lichter Date 10/26/04

NOTE: All materials must be submitted to and approved by the Village Manager's Office by 12:00 noon, Wednesday, prior to the Agenda Distribution.

MEMORANDUM

TO: William T. Lichter, Village Manager

FROM: David A. Hulseberg, AICP, Director of Community Development DAH

DATE: October 25, 2004

SUBJECT: YORKTOWN CERTIFICATION LETTER

Attached please find correspondence to Long-Pehrson Associates relative to the Purchase and Sale Agreement for the hotel/convention center. Long-Pehrson is requesting this correspondence as part of the sales transaction of that property and to confirm the Village's assistance for future development activity for the Yorktown Mall property itself.

The salient points of this correspondence are:

a. The Village agrees to assist the Owner in future stormwater applications for relief from the Village and County Ordinances for stormwater improvements.
b. The Village agrees to grant a fee in lieu of detention if the County grants a variation from their Ordinance.
c. The Village agrees to reimburse the Owner from their Fee In Lieu of Detention (FILO) payment for certain expenditures that provide stormwater or water quality improvements.
d. Owner may take a fee in lieu of detention credit for stormwater improvements made to the existing detention pond.

Recommendation:
Staff recommends that the Village Board authorize the Village Manager to sign the Yorktown Certification Letter dated November 4, 2004 on behalf of the Village of Lombard.

/jd

h:\cd\worduser\director\hotel&conventioncenter\botitems\botmemoyorktowncertificationletter

VILLAGE OF LOMBARD
255 E. Wilson Ave.
Lombard, Illinois 60148
630/620-5700 FAX: 630/620-8222
TDD: 630/620-5812
www.villageoflombard.org

October 14, 2004

Mr. Robert Long
Long-Pehrson Associates, LLC
203 Yorktown Shopping Center
Lombard, IL 60148

Re: Yorktown Certification Letter

Dear Mr. Long:

In reference to Section 7.3(c) of the Purchase and Sale Agreement between Yorktown and the Harp Group, this letter serves to clarify the stormwater detention rights enjoyed by the Yorktown Mall Planned Development and its peripheral Planned Development.

As stated in previous meetings between the Village of Lombard staff, Yorktown counsel, and Yorktown Mall representatives, the Village of Lombard is a partial waiver community and must adhere to the DuPage County Stormwater regulations. Further, the Village of Lombard has developed a companion set of stormwater ordinances found in the Village of Lombard Code of Ordinances, Title 15, Section 151. The Village's partial waiver status provides that the municipality may review stormwater plans on behalf of the County. If those plans meet the ordinance, the Village has the authority to approve such plans. If the plans do not meet the terms of the County Ordinance, the Village must direct applicant to the County for a variation. The Village is subject to County review of its actions. The Village can be fined and lose its partial waiver status if it fails to enforce the County Ordinance.

The major difference between the Village's and the County's stormwater ordinances is that the Village's Ordinance applies to all commercial redevelopment and additions to commercial property. Both the Village and the County have variation processes for relief when the stormwater ordinances cannot be met. The variation process on both the County and Village level can grant relief to allow a fee in lieu of detention instead of on-site detention.

In both the County and Village variation instances, if a fee in lieu of detention is approved the Village of Lombard receives the payment and those monies are held in a watershed fund for that respective area. These watershed funds can be used by the Village or their designee for stormwater improvements to that watershed to enhance existing site runoff storage facilities and related

components, construct off-site facilities and related components, provide maintenance of stormwater facilities, or undertake other development that provide a benefit to the specific watershed.

Yorktown's stormwater consultant prepared a report dated January 27, 1997 and last revised on May 20, 1997 identifying the volume of stormwater capacity available in the detention pond and for which properties they are provided for (a copy of that report including a drainage tributary map is attached). A total of 13.19 ac-ft was required for the proposed and future developments on the mall's eastern perimeter lots as follows:

Area	Pervious Area	Impervious Area	Total Area	% Impervious
Lot 2	0.98	5.71	6.69	85.4
Lot 4	1.02	4.87	5.89	82.7
Lot 5	0.64	2.88	3.52	81.8
Lot 6	0.94	2.71	3.65	74.2
Cinema	0.92	7.00	7.92	88.4
Firestone	0.26	0.64	0.90	71.1
Ring Road	0.21	0.66	0.87	75.9

The following table adds the volumes of more recent analyses for lots 2 and 4.

Existing volume provided for the Eastern Lots including the Cinema:	13.19 ac-ft
Additional volume required for hotel/convention center on Lot 2:	0.10 ac-ft
Additional volume required for widening of Convention Way:	0.40 ac-ft
Additional volume required for 95% imperviousness on Lot 4:	0.24 ac-ft
TOTAL VOLUME REQUIRED FOR ALL EASTERN LOTS =	13.93 ac-ft

Based on the volumes above, the Lombard Public Facility Corporation will be responsible to provide for 0.50 ac-ft of storage as a result of the hotel and convention center project. (0.10 ac-ft for Lot 2 and 0.40 ac-ft for Convention Way). Yorktown Mall has no additional capacity at the detention pond.

In regard to stormwater requirements applicable to the Yorktown Mall Planned Development (hereinafter the "Subject Property"), the Village has committed to the following:

1. Upon a request from the Owner, the Village hereby agrees to assist the Owner in future applications for relief from the Village's Flood Control Ordinance as well as any requisite relief from the provisions of the DuPage County Countywide Stormwater and Flood Plain Ordinance (hereinafter referred to as the Countywide Ordinance), as administered by DuPage County, for the following improvements on the Subject Property:

a. Any stormwater improvements that would require the creation of additional stormwater capacity associated with the construction of a lifestyle center on the former Montgomery Ward property, and
b. Any stormwater improvements that would require the creation of additional stormwater capacity associated with the construction of future outlots on the Subject Property.

The assistance from the Village shall be subject to the terms and conditions as expressed herein.

2. Should DuPage County grant a variation from the Countywide Ordinance, the Village agrees to grant a fee in lieu of detention variance, as provided for in Section 151.57 of the Village's Flood Control Ordinance.

3. For any fees that are collected as part of the variation request, the Village hereby agrees to reimburse the Owner for the following expenditures that provide stormwater detention or water quality improvements:
a. Any storm sewer improvements on the Subject Property;
b. Any improvements to existing detention or retention facilities on the Subject Property that improve water quality, as determined by the Village's Engineer; or
c. Any improvements that increase the available stormwater detention capacity on the Subject Property.

4. The Owner and the Village hereby agree that the Owner may take a fee in lieu of detention credit for the proposed $300,000 in stormwater improvements being made to the existing detention pond on the Subject Property as part of the hotel development to be constructed on a portion of the Subject Property. The $300,000 cost of said stormwater improvements would be considered credited as fee in lieu of detention once those improvements are completed.

Sincerely,

VILLAGE OF LOMBARD

William T. Lichter
Village Manager

DAH/jd

cc: Len Flood, Director of Finance
Thomas P. Bayer, Village Attorney
David A. Hulseberg, AICP, Director of Community Development
Thomas McGuigan, Harp Mid-America LLC, 111 S. Lincoln, Hinsdale, IL 60521
Jim Romano, 203 Yorktown, Lombard, IL 60148

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VILLAGE OF LOMBARD
REQUEST FOR BOARD OF TRUSTEES ACTION
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_____ Resolution or Ordinance (Blue) _____ *Waiver of First Requested*
_____ Recommendations of Boards, Commissions & Committees (Green)
__X__ Other Business (Pink)

TO: PRESIDENT AND BOARD OF TRUSTEES

FROM: William T. Lichter, Village Manager

DATE: October 25, 2004 (B of T) Date: November 4, 2004

TITLE: Yorktown Shopping Center Certification Letter

SUBMITTED BY: Department of Community Development

BACKGROUND/POLICY IMPLICATIONS:

The Department of Community Development transmits for your consideration correspondence to Long-Pehrson Associates relative to future stormwater improvements associated with the Yorktown Planned Development and its peripheral Planned Development. (DISTRICT #3)

Staff recommends approval of this request.

Please place this item on the November 4, 2004 Board of Trustees agenda.

Fiscal Impact/Funding Source:

Review (as necessary):
Village Attorney X_____ Date _____
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Village Manager X W. Thomas Lichter Date 10/26/04

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MEMORANDUM

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FROM: David A. Hulseberg, AICP, Director of Community Development DAH

DATE: October 25, 2004

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Recommendation:
Staff recommends that the Village Board authorize the Village Manager to sign the Yorktown Certification Letter dated November 4, 2004 on behalf of the Village of Lombard.

/jd

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VILLAGE OF LOMBARD

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Village Manager

DAH/jd

cc: Len Flood, Director of Finance
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Thomas McGuigan, Harp Mid-America LLC, 111 S. Lincoln, Hinsdale, IL 60521
Jim Romano, 203 Yorktown, Lombard, IL 60148

STORMWATER SUBMITTAL

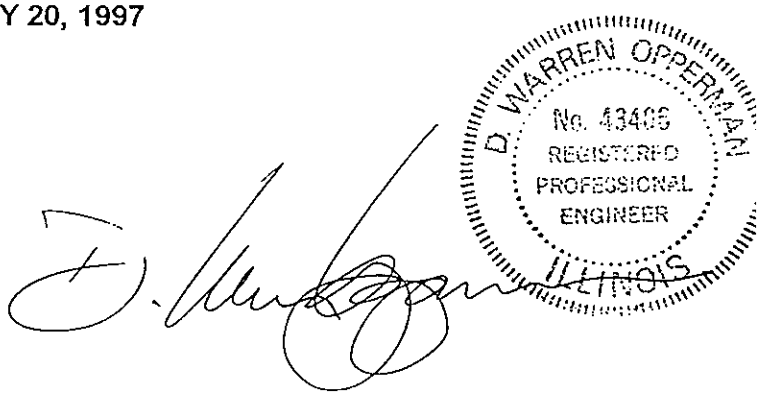
YORKTOWN CINEMA REDEVELOPMENT

YORKTOWN PLAZA
LOMBARD, ILLINOIS

JOSEPH A. SCHUDT & ASSOCIATES
19350 SOUTH HARLEM AVENUE
FRANKFORT, IL 60423

JAS #96-74

JANUARY 27, 1997
REVISED MARCH 20, 1997
REVISED MAY 20, 1997



Quick TR-55 Ver.5.46 S/N:
Executed: 13:27:20 01-24-1997

LOMBARD YORKTOWN
SOUTH PARCELS + CINEMA REDEVELOPMENT

RUNOFF CURVE NUMBER DATA

Composite Area:

SURFACE DESCRIPTION	AREA (acres)	CN	
PAVING,BUILDING,WATER	24.47	98	
LANDSCAPE,GOOD,C	4.97	74	
COMPOSITE AREA --->	29.44	93.9	(94)

S/N:

LOMBARD YORKTOWN
SOUTH PARCELS

CALCULATED 03-20-1997 13:46:47
DISK FILE: 9262-S .VOL

Planimeter scale: 1 inch = 208.71 ft.

Elevation (ft)	Planimeter (sq.in.)	Area (acres)	A1+A2+sqr(A1*A2) (acres)	* Volume (acre-ft)	Volume Sum (acre-ft)
724.00	1.96	1.96	0.00	0.00	0.00
725.00	2.19	2.19	6.22	2.07	2.07
726.00	2.36	2.36	6.82	2.27	4.35
727.00	2.53	2.53	7.33	2.44	6.79
728.00	2.70	2.70	7.84	2.61	9.41
729.00	2.88	2.88	8.37	2.79	12.20
730.00	3.06	3.06	8.91	2.97	15.17
731.00	3.24	3.24	9.45	3.15	18.32

* Incremental volume computed by the Conic Method for Reservoir Volumes.

Pond File: 9262-S .PND
Inflow Hydrograph: 9262S100.HYD
Outflow Hydrograph: 9262SOUT.HYD

INFLOW HYDROGRAPH		ROUTING COMPUTATIONS				
TIME (hrs)	INFLOW (cfs)	I1+I2 (cfs)	2S/t - O (cfs)	2S/t + O (cfs)	OUTFLOW (cfs)	ELEVATION (ft)
11.000	7.00	-----	-2.9	2.9	2.94	724.00
11.100	8.00	15.0	6.2	12.1	2.94	724.02
11.200	9.00	17.0	17.3	23.2	2.94	724.04
11.300	10.00	19.0	30.4	36.3	2.94	724.07
11.400	12.00	22.0	46.5	52.4	2.94	724.10
11.500	13.00	25.0	65.7	71.5	2.94	724.14
11.600	15.00	28.0	87.8	93.7	2.94	724.18
11.700	32.00	47.0	128.9	134.8	2.94	724.26
11.800	49.00	81.0	204.0	209.9	2.94	724.41
11.900	66.00	115.0	313.1	319.0	2.94	724.63
12.000	127.00	193.0	500.3	506.1	2.94	725.00
12.100	233.00	360.0	854.4	860.3	2.94	725.65
12.200	252.00	485.0	1333.5	1339.4	2.94	726.48
12.300	152.00	404.0	1731.6	1737.5	2.94	727.14
12.400	79.00	231.0	1956.7	1962.6	2.94	727.50
12.500	52.00	131.0	2081.9	2087.7	2.94	727.70
12.600	40.00	92.0	2168.0	2173.9	2.94	727.83
12.700	32.00	72.0	2234.1	2240.0	2.94	727.94
12.800	27.00	59.0	2287.2	2293.1	2.94	728.02
12.900	24.00	51.0	2332.3	2338.2	2.94	728.09
13.000	22.00	46.0	2372.5	2378.3	2.94	728.15
13.100	20.00	42.0	2408.6	2414.5	2.94	728.20
13.200	19.00	39.0	2441.7	2447.6	2.94	728.25
13.300	18.00	37.0	2472.8	2478.7	2.94	728.29
13.400	17.00	35.0	2501.9	2507.8	2.94	728.34
13.500	16.00	33.0	2529.1	2534.9	2.94	728.38
13.600	15.00	31.0	2554.2	2560.1	2.94	728.41
13.700	14.00	29.0	2577.3	2583.2	2.94	728.45
13.800	14.00	28.0	2599.4	2605.3	2.94	728.48
13.900	14.00	28.0	2621.5	2627.4	2.94	728.51
14.000	13.00	27.0	2642.7	2648.5	2.94	728.55
14.100	12.00	25.0	2661.8	2667.7	2.94	728.57
14.200	12.00	24.0	2679.9	2685.8	2.94	728.60
14.300	11.00	23.0	2697.0	2702.9	2.94	728.63
14.400	11.00	22.0	2713.1	2719.0	2.94	728.65
14.500	10.00	21.0	2728.3	2734.1	2.94	728.67
14.600	10.00	20.0	2742.4	2748.3	2.94	728.69
14.700	10.00	20.0	2756.5	2762.4	2.94	728.71
14.800	9.00	19.0	2769.6	2775.5	2.94	728.73
14.900	9.00	18.0	2781.7	2787.6	2.94	728.75
15.000	9.00	18.0	2793.9	2799.7	2.94	728.77
15.100	9.00	18.0	2806.0	2811.9	2.94	728.79
15.200	9.00	18.0	2818.1	2824.0	2.94	728.81
15.300	9.00	18.0	2830.2	2836.1	2.94	728.82
15.400	9.00	18.0	2842.3	2848.2	2.94	728.84

Pond File: 9262-S .PND
Inflow Hydrograph: 9262S100.HYD
Outflow Hydrograph: 9262SOUT.HYD

INFLOW HYDROGRAPH		ROUTING COMPUTATIONS				
TIME (hrs)	INFLOW (cfs)	I1+I2 (cfs)	2S/t - O (cfs)	2S/t + O (cfs)	OUTFLOW (cfs)	ELEVATION (ft)
20.100	4.00	8.0	3137.0	3142.9	2.94	729.26
20.200	4.00	8.0	3139.1	3145.0	2.94	729.26
20.300	4.00	8.0	3141.2	3147.1	2.94	729.27
20.400	4.00	8.0	3143.3	3149.2	2.94	729.27
20.500	4.00	8.0	3145.5	3151.3	2.94	729.27
20.600	4.00	8.0	3147.6	3153.5	2.94	729.28
20.700	4.00	8.0	3149.7	3155.6	2.94	729.28
20.800	4.00	8.0	3151.8	3157.7	2.94	729.28
20.900	4.00	8.0	3153.9	3159.8	2.94	729.28
21.000	4.00	8.0	3156.1	3161.9	2.94	729.29
21.100	4.00	8.0	3158.2	3164.1	2.94	729.29
21.200	4.00	8.0	3160.3	3166.2	2.94	729.29
21.300	4.00	8.0	3162.4	3168.3	2.94	729.30
21.400	4.00	8.0	3164.5	3170.4	2.94	729.30
21.500	4.00	8.0	3166.7	3172.5	2.94	729.30
21.600	4.00	8.0	3168.8	3174.7	2.94	729.31
21.700	4.00	8.0	3170.9	3176.8	2.94	729.31
21.800	4.00	8.0	3173.0	3178.9	2.94	729.31
21.900	4.00	8.0	3175.1	3181.0	2.94	729.31
22.000	4.00	8.0	3177.3	3183.1	2.94	729.32
22.100	4.00	8.0	3179.4	3185.3	2.94	729.32
22.200	4.00	8.0	3181.5	3187.4	2.94	729.32
22.300	4.00	8.0	3183.6	3189.5	2.94	729.33
22.400	4.00	8.0	3185.7	3191.6	2.94	729.33
22.500	4.00	8.0	3187.9	3193.7	2.94	729.33
22.600	3.00	7.0	3189.0	3194.9	2.94	729.33
22.700	3.00	6.0	3189.1	3195.0	2.94	729.33
22.800	3.00	6.0	3189.2	3195.1	2.94	729.33
22.900	3.00	6.0	3189.3	3195.2	2.94	729.33
23.000	3.00	6.0	3189.5	3195.3	2.94	729.33
23.100	3.00	6.0	3189.6	3195.5	2.94	729.33
23.200	3.00	6.0	3189.7	3195.6	2.94	729.33
23.300	3.00	6.0	3189.8	3195.7	2.94	729.33
23.400	3.00	6.0	3189.9	3195.8	2.94	729.33
23.500	2.00	5.0	3189.1	3194.9	2.94	729.33
23.600	2.00	4.0	3187.2	3193.1	2.94	729.33
23.700	2.00	4.0	3185.3	3191.2	2.94	729.33
23.800	2.00	4.0	3183.4	3189.3	2.94	729.33
23.900	2.00	4.0	3181.5	3187.4	2.94	729.32
24.000	2.00	4.0	3179.7	3185.5	2.94	729.32
24.100	2.00	4.0	3177.8	3183.7	2.94	729.32
24.200	2.00	4.0	3175.9	3181.8	2.94	729.32
24.300	2.00	4.0	3174.0	3179.9	2.94	729.31
24.400	2.00	4.0	3172.1	3178.0	2.94	729.31
24.500	2.00	4.0	3170.3	3176.1	2.94	729.31
24.600	1.00	3.0	3167.4	3173.3	2.94	729.30

********** SUMMARY OF ROUTING COMPUTATIONS **********

Pond File: 9262-S .PND
Inflow Hydrograph: 9262S100.HYD
Outflow Hydrograph: 9262SOUT.HYD

Starting Pond W.S. Elevation = 724.00 ft

***** Summary of Peak Outflow and Peak Elevation *****

Peak Inflow	=	252.00 cfs
Peak Outflow	=	2.94 cfs
Peak Elevation	=	729.33 ft

***** Summary of Approximate Peak Storage *****

Initial Storage	=	0.00 ac-ft
Peak Storage From Storm	=	13.19 ac-ft
Total Storage in Pond	=	13.19 ac-ft

Warning: Inflow hydrograph truncated on left side.

```
14.0 - | X *
       | X*
14.1 - | X*
       | X*
14.2 - | X*
       | X*
14.3 - | X*
       | X*
14.4 - | X*
       | X*
14.5 - | X*
       | X*
14.6 - | X*
       | X*
14.7 - | X*
       | X*
14.8 - | X*
       | X*
14.9 - | X*
       | X*
15.0 - | X*
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15.1 - | X*
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15.2 - | X*
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15.3 - | X*
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15.4 - | X*
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15.5 - | X*
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15.6 - | X*
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15.7 - | X*
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15.8 - | X*
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15.9 - | X*
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16.0 - | X*
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16.1 - | X*
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16.2 - | X*
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16.3 - | X
       | X
16.4 - | X
       | X
16.5 - | X
       | X
16.6 - | X
       | X
16.7 - | X
       | X
16.8 - | X
       | X
16.9 - | X
       | X
17.0 - | X
       | X
17.1 - | X
       | X
17.2 - | X
```

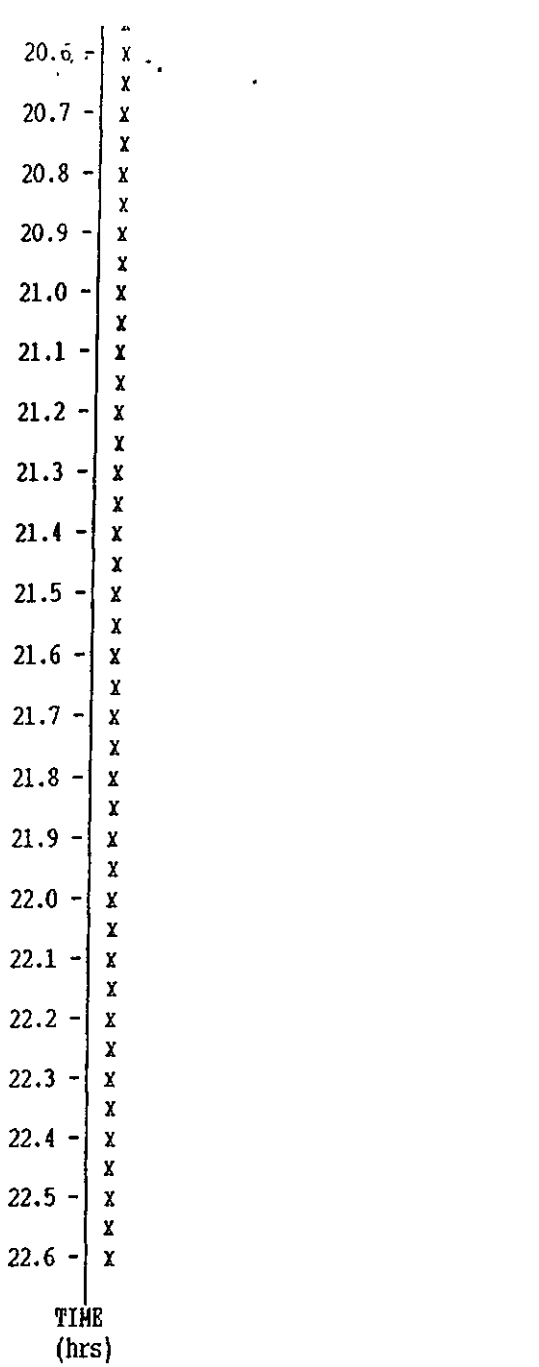

* File: 9262S100.HYD Qmax = 252.0 cfs
x File: 9262SOUT.HYD Qmax = 2.9 cfs

HYDRAULIC REPORT FOR

YORKTOWN CINEMA REDEVLPMNT

LOMBARD, ILLINOIS

10 YEAR DESIGN STORM

JOSEPH A. SCHUDT & ASSOC.

19350 S. HARLEM AVE.

FRANKFORT, IL. 60423

MAY 20, 1997

JOB # 96-74

LINE 3 / Q = 53.33 / HT = 48 / WID = 48 / N = .013 / L = 140 / JLC = 0

C-D / DNLN = 2

	HGL	DEPTH	INVERT	VEL	EGL	T WID	COVER	AREA
DNSTRM	729.12	36.03	726.12	5.27	729.55	41.59	2.08	10.12
UPSTRM	729.36	36.54	726.32	5.20	729.78	40.93	1.18	10.26

Drainage area (ac)	= 1.50	Slope of invert (%)	= 0.1400
Runoff coefficient	= 0.90	Slope energy grade line (%)	= 0.1615
Time of conc (min)	= 11.29	Critical depth (in)	= 25.91
Inlet time (min)	= 7.00	Natural ground elev. (ft)	= 731.50
Intensity (in/hr)	= 5.67	Upstream surcharge (ft)	= 0.00
Cumulative C*A	= 9.40	Additional Q (cfs)	= 0.00
Q = CA * I (cfs)	= 53.33	Line capacity (cfs)	= 53.75
Q catchment (cfs)	= 8.83	Inlet length (ft)	= 300.00
Q carryover (cfs)	= 0.00	Gutter slope (ft/ft)	= 0.0100
Q captured (cfs)	= 8.83	Cross slope (ft/ft)	= 0.0100
Q bypassed (cfs)	= 0.00	Ponding width (ft)	= N/A

LINE 4 / Q = 46.68 / HT = 48 / WID = 48 / N = .013 / L = 125 / JLC = 0

D-E / DNLN = 3

	HGL	DEPTH	INVERT	VEL	EGL	T WID	COVER	AREA
DNSTRM	729.36	36.61	726.31	4.54	729.68	41.92	1.19	10.28
UPSTRM	729.50	36.75	726.44	4.52	729.82	40.67	1.05	10.32

Drainage area (ac)	= 1.62	Slope of invert (%)	= 0.1040
Runoff coefficient	= 0.90	Slope energy grade line (%)	= 0.1113
Time of conc (min)	= 10.60	Critical depth (in)	= 24.24
Inlet time (min)	= 8.00	Natural ground elev. (ft)	= 731.50
Intensity (in/hr)	= 5.79	Upstream surcharge (ft)	= 0.00
Cumulative C*A	= 8.05	Additional Q (cfs)	= 0.00
Q = CA * I (cfs)	= 46.68	Line capacity (cfs)	= 46.33
Q catchment (cfs)	= 9.21	Inlet length (ft)	= 350.00
Q carryover (cfs)	= 0.00	Gutter slope (ft/ft)	= 0.0100
Q captured (cfs)	= 9.21	Cross slope (ft/ft)	= 0.0100
Q bypassed (cfs)	= 0.00	Ponding width (ft)	= N/A

LINE 7 / Q = 0.89 / HT = 12 / WID = 12 / N = .013 / L = 100 / JLC = 0

G-H / DNLN = 6

	HGL	DEPTH	INVERT	VEL	EGL	T WID	COVER	AREA
DNSTRM	730.09	12.00	727.50	1.13	730.11	0.00	2.5	0.79
UPSTRM	730.15	12.00	728.00	1.13	730.17	0.00	1.5	0.79

Drainage area (ac)	= 0.14	Slope of invert (%)	= 0.5000
Runoff coefficient	= 0.90	Slope energy grade line (%)	= 0.0621
Time of conc (min)	= 5.00	Critical depth (in)	= 4.79
Inlet time (min)	= 5.00	Natural ground elev. (ft)	= 730.50
Intensity (in/hr)	= 7.05	Upstream surcharge (ft)	= 1.15
Cumulative C*A	= 0.13	Additional Q (cfs)	= 0.00
Q = CA * I (cfs)	= 0.89	Line capacity (cfs)	= 2.52
Q catchment (cfs)	= 0.89	Inlet length (ft)	= 80.00
Q carryover (cfs)	= 0.00	Gutter slope (ft/ft)	= 0.0100
Q captured (cfs)	= 0.89	Cross slope (ft/ft)	= 0.0100
Q bypassed (cfs)	= 0.00	Ponding width (ft)	= N/A

LINE 8 / Q = 20.48 / HT = 24 / WID = 24 / N = .013 / L = 185 / JLC = 0

E-I / DNLN = 4

	HGL	DEPTH	INVERT	VEL	EGL	T WID	COVER	AREA
DNSTRM	729.60	19.23	728.00	7.59	730.50	21.48	1.5	2.70
UPSTRM	731.19	19.88	729.53	7.36	732.03	18.10	3.46	2.78

Drainage area (ac)	= 0.65	Slope of invert (%)	= 0.8270
Runoff coefficient	= 0.90	Slope energy grade line (%)	= 0.8275
Time of conc (min)	= 9.57	Critical depth (in)	= 19.23
Inlet time (min)	= 6.00	Natural ground elev. (ft)	= 735.00
Intensity (in/hr)	= 5.99	Upstream surcharge (ft)	= 0.00
Cumulative C*A	= 3.42	Additional Q (cfs)	= 0.00
Q = CA * I (cfs)	= 20.48	Line capacity (cfs)	= 20.57
Q catchment (cfs)	= 3.97	Inlet length (ft)	= 250.00
Q carryover (cfs)	= 0.00	Gutter slope (ft/ft)	= 0.0120
Q captured (cfs)	= 3.97	Cross slope (ft/ft)	= 0.0120
Q bypassed (cfs)	= 0.00	Ponding width (ft)	= N/A

LINE 11 / Q = 14.49 / HT = 18 / WID = 18 / N = .013 / L = 25 / JLC = 0

J1-K / DNLN = 10

	HGL	DEPTH	INVERT	VEL	EGL	T WID	COVER	AREA
DNSTRM	733.65	18.00	732.10	8.20	734.69	0.00	3.4	1.77
UPSTRM	734.07	17.88	732.58	8.21	735.11	2.90	2.92	1.77

Drainage area (ac)	= 0.25	Slope of invert (%)	= 1.9001
Runoff coefficient	= 0.90	Slope energy grade line (%)	= 1.6712
Time of conc (min)	= 7.77	Critical depth (in)	= 16.93
Inlet time (min)	= 6.00	Natural ground elev. (ft)	= 737.00
Intensity (in/hr)	= 6.37	Upstream surcharge (ft)	= 0.00
Cumulative C*A	= 2.28	Additional Q (cfs)	= 0.00
Q = CA * I (cfs)	= 14.49	Line capacity (cfs)	= 14.48
Q catchment (cfs)	= 1.53	Inlet length (ft)	= 180.00
Q carryover (cfs)	= 0.00	Gutter slope (ft/ft)	= 0.0090
Q captured (cfs)	= 1.53	Cross slope (ft/ft)	= 0.0150
Q bypassed (cfs)	= 0.00	Ponding width (ft)	= N/A

LINE 12 / Q = 13.20 / HT = 18 / WID = 18 / N = .013 / L = 53 / JLC = 0

K-L / DNLN = 11

	HGL	DEPTH	INVERT	VEL	EGL	T WID	COVER	AREA
DNSTRM	734.07	17.94	732.57	7.47	734.93	17.97	2.92	1.77
UPSTRM	734.92	18.00	733.42	7.47	735.78	0.00	2.48	1.77

Drainage area (ac)	= 0.43	Slope of invert (%)	= 1.6000
Runoff coefficient	= 0.90	Slope energy grade line (%)	= 1.6080
Time of conc (min)	= 7.47	Critical depth (in)	= 16.47
Inlet time (min)	= 6.00	Natural ground elev. (ft)	= 737.40
Intensity (in/hr)	= 6.43	Upstream surcharge (ft)	= 0.00
Cumulative C*A	= 2.05	Additional Q (cfs)	= 0.00
Q = CA * I (cfs)	= 13.20	Line capacity (cfs)	= 13.28
Q catchment (cfs)	= 2.63	Inlet length (ft)	= 180.00
Q carryover (cfs)	= 0.00	Gutter slope (ft/ft)	= 0.0090
Q captured (cfs)	= 2.63	Cross slope (ft/ft)	= 0.0150
Q bypassed (cfs)	= 0.00	Ponding width (ft)	= N/A

HYDRAULIC REPORT FOR

YORKTOWN CINEMA REDEVLPMNT

LOMBARD, ILLINOIS

100 YEAR DESIGN STORM

JOSEPH A. SCHUDT & ASSOC.

19350 S. HARLEM AVE.

FRANKFORT, IL. 60423

MAY 20, 1997

JOB # 96-74

LINE 3 / Q = 82.46 / HT = 48 / WID = 48 / N = .013 / L = 140 / JLC = 0

C-D / DNLN = 2

	HGL	DEPTH	INVERT	VEL	EGL	T WID	COVER	AREA
DNSTRM	730.09	47.59	726.12	6.57	730.76	47.80	2.08	12.55
UPSTRM	730.53	48.00	726.32	6.56	731.20	0.00	1.18	12.57

Drainage area (ac)	= 1.50	Slope of invert (%)	= 0.1400
Runoff coefficient	= 0.90	Slope energy grade line (%)	= 0.3175
Time of conc (min)	= 11.29	Critical depth (in)	= 32.22
Inlet time (min)	= 7.00	Natural ground elev. (ft)	= 731.50
Intensity (in/hr)	= 8.77	Upstream surcharge (ft)	= 0.22
Cumulative C*A	= 9.40	Additional Q (cfs)	= 0.00
Q = CA * I (cfs)	= 82.46	Line capacity (cfs)	= 53.75
Q catchment (cfs)	= 13.66	Inlet length (ft)	= 300.00
Q carryover (cfs)	= 0.00	Gutter slope (ft/ft)	= 0.0100
Q captured (cfs)	= 13.66	Cross slope (ft/ft)	= 0.0100
Q bypassed (cfs)	= 0.00	Ponding width (ft)	= N/A

LINE 4 / Q = 72.18 / HT = 48 / WID = 48 / N = .013 / L = 125 / JLC = 0

D-E / DNLN = 3

	HGL	DEPTH	INVERT	VEL	EGL	T WID	COVER	AREA
DNSTRM	730.53	48.00	726.31	5.75	731.04	0.00	1.19	12.56
UPSTRM	730.85	48.00	726.44	5.74	731.36	0.00	1.05	12.57

Drainage area (ac)	= 1.62	Slope of invert (%)	= 0.1040
Runoff coefficient	= 0.90	Slope energy grade line (%)	= 0.2526
Time of conc (min)	= 10.60	Critical depth (in)	= 30.15
Inlet time (min)	= 8.00	Natural ground elev. (ft)	= 731.50
Intensity (in/hr)	= 8.96	Upstream surcharge (ft)	= 0.41
Cumulative C*A	= 8.05	Additional Q (cfs)	= 0.00
Q = CA * I (cfs)	= 72.18	Line capacity (cfs)	= 46.33
Q catchment (cfs)	= 14.24	Inlet length (ft)	= 350.00
Q carryover (cfs)	= 0.00	Gutter slope (ft/ft)	= 0.0100
Q captured (cfs)	= 14.24	Cross slope (ft/ft)	= 0.0100
Q bypassed (cfs)	= 0.00	Ponding width (ft)	= N/A

LINE 7 / Q = 1.37 / HT = 12 / WID = 12 / N = .013 / L = 100 / JLC = 0

G-H / DNLN = 6

	HGL	DEPTH	INVERT	VEL	EGL	T WID	COVER	AREA
DNSTRM	732.25	12.00	727.50	1.75	732.30	0.00	2.5	0.79
UPSTRM	732.40	12.00	728.00	1.75	732.45	0.00	1.5	0.79

Drainage area (ac)	= 0.14	Slope of invert (%)	= 0.5000
Runoff coefficient	= 0.90	Slope energy grade line (%)	= 0.1491
Time of conc (min)	= 5.00	Critical depth (in)	= 5.97
Inlet time (min)	= 5.00	Natural ground elev. (ft)	= 730.50
Intensity (in/hr)	= 10.91	Upstream surcharge (ft)	= 3.40
Cumulative C*A	= 0.13	Additional Q (cfs)	= 0.00
Q = CA * I (cfs)	= 1.37	Line capacity (cfs)	= 2.52
Q catchment (cfs)	= 1.37	Inlet length (ft)	= 80.00
Q carryover (cfs)	= 0.00	Gutter slope (ft/ft)	= 0.0100
Q captured (cfs)	= 1.37	Cross slope (ft/ft)	= 0.0100
Q bypassed (cfs)	= 0.00	Ponding width (ft)	= N/A

LINE 8 / Q = 31.68 / HT = 24 / WID = 24 / N = .013 / L = 185 / JLC = 0

E-I / DNLN = 4

	HGL	DEPTH	INVERT	VEL	EGL	T WID	COVER	AREA
DNSTRM	730.85	24.00	728.00	10.09	732.43	0.00	1.5	3.14
UPSTRM	734.48	24.00	729.53	10.08	736.06	0.00	3.46	3.14

Drainage area (ac)	= 0.65	Slope of invert (%)	= 0.8270
Runoff coefficient	= 0.90	Slope energy grade line (%)	= 1.9625
Time of conc (min)	= 9.57	Critical depth (in)	= 22.85
Inlet time (min)	= 6.00	Natural ground elev. (ft)	= 735.00
Intensity (in/hr)	= 9.26	Upstream surcharge (ft)	= 2.95
Cumulative C*A	= 3.42	Additional Q (cfs)	= 0.00
Q = CA * I (cfs)	= 31.68	Line capacity (cfs)	= 20.57
Q catchment (cfs)	= 6.14	Inlet length (ft)	= 250.00
Q carryover (cfs)	= 0.00	Gutter slope (ft/ft)	= 0.0120
Q captured (cfs)	= 6.14	Cross slope (ft/ft)	= 0.0120
Q bypassed (cfs)	= 0.00	Ponding width (ft)	= N/A

LINE 11 / Q = 22.43 / HT = 18 / WID = 18 / N = .013 / L = 25 / JLC = 0

J1-K / DNLN = 10

	HGL	DEPTH	INVERT	VEL	EGL	T WID	COVER	AREA
DNSTRM	740.60	18.00	732.10	12.69	743.10	0.00	3.4	1.77
UPSTRM	741.74	18.00	732.58	12.69	744.24	0.00	2.92	1.77

Drainage area (ac)	= 0.25	Slope of invert (%)	= 1.9001
Runoff coefficient	= 0.90	Slope energy grade line (%)	= 4.5619
Time of conc (min)	= 7.77	Critical depth (in)	= 17.84
Inlet time (min)	= 6.00	Natural ground elev. (ft)	= 737.00
Intensity (in/hr)	= 9.85	Upstream surcharge (ft)	= 7.67
Cumulative C*A	= 2.28	Additional Q (cfs)	= 0.00
Q = CA * I (cfs)	= 22.43	Line capacity (cfs)	= 14.48
Q catchment (cfs)	= 2.36	Inlet length (ft)	= 180.00
Q carryover (cfs)	= 0.00	Gutter slope (ft/ft)	= 0.0090
Q captured (cfs)	= 2.36	Cross slope (ft/ft)	= 0.0150
Q bypassed (cfs)	= 0.00	Ponding width (ft)	= N/A

LINE 12 / Q = 20.42 / HT = 18 / WID = 18 / N = .013 / L = 53 / JLC = 0

K-L / DNLN = 11

	HGL	DEPTH	INVERT	VEL	EGL	T WID	COVER	AREA
DNSTRM	741.74	18.00	732.57	11.56	743.82	0.00	2.92	1.77
UPSTRM	743.75	18.00	733.42	11.56	745.82	0.00	2.48	1.77

Drainage area (ac)	= 0.43	Slope of invert (%)	= 1.6000
Runoff coefficient	= 0.90	Slope energy grade line (%)	= 3.7827
Time of conc (min)	= 7.47	Critical depth (in)	= 17.74
Inlet time (min)	= 6.00	Natural ground elev. (ft)	= 737.40
Intensity (in/hr)	= 9.95	Upstream surcharge (ft)	= 8.83
Cumulative C*A	= 2.05	Additional Q (cfs)	= 0.00
Q = CA * I (cfs)	= 20.42	Line capacity (cfs)	= 13.28
Q catchment (cfs)	= 4.06	Inlet length (ft)	= 180.00
Q carryover (cfs)	= 0.00	Gutter slope (ft/ft)	= 0.0090
Q captured (cfs)	= 4.06	Cross slope (ft/ft)	= 0.0150
Q bypassed (cfs)	= 0.00	Ponding width (ft)	= N/A

YORKTOWN CINEMA REDEVELOPMENT
INLET GRATE CAPACITIES

96-74

TYPE M1 GRATE (EJIW 1022-2)

OPENING AREA = 120 IN^2 = 0.833 FT^2

$Q = CA\sqrt{2gh}$ h = 1 FT MAX FLOOD DEPTH

C = 0.61 FOR SHARP EDGE

$Q = 0.61(.833)\sqrt{(64.4)(1)}$

$Q_{MAX} = (0.61)(.833)(8.025)$ = 4.08 CFS / STRUCTU

TYPE 5340 STRUCTURE FRAME (EJIW)

OPENING AREA = 190 IN^2 = 1.319 FT^2

$Q = CA\sqrt{2gh}$

$Q = 0.61(1.319)\sqrt{64.4(1)}$

$= (0.61)(1.319)(8.025)$ = 6.456 CFS / STRUCTURE

Culvert Designer/Analyzer Report
YORKTOWN - RT 56

Analysis Component			
Storm Event	Design	Discharge	12.74 cfs

Peak Discharge Method: SCS Peak Discharge			
Design Rainfall Depth	6.46 in	Check Rainfall Depth	0.00 in
Design Peak Discharge	12.74 cfs	Check Peak Discharge	0.00 cfs
Total Area	5.50 acres	Time of Concentration	60.00 min
Weighted Curve Number	80	Rainfall Type	Type II

Subwatershed	Area (acres)	CN
1	2.75	98
2	2.75	61

Tailwater Conditions: Constant Tailwater	
Tailwater Elevation	0.00 ft

Name	Description	Discharge	HW Elev	Velocity
Weir	Not Considered	N/A	N/A	N/A